

Principal Stress & Maximal Shear Stress

2-Dimensional

$$\sigma_{1,2} = \frac{\sigma_x + \sigma_y}{2} \pm \sqrt{\left(\frac{\sigma_x - \sigma_y}{2}\right)^2 + \tau_{xy}^2}$$

$$\tau_{max} = \sqrt{\left(\frac{\sigma_x - \sigma_y}{2}\right)^2 + \tau_{xy}^2}$$

$$\sigma_{x'} = \frac{\sigma_x + \sigma_y}{2} + \left(\frac{\sigma_x - \sigma_y}{2}\right) \cos 2\theta + \tau_{xy} \sin 2\theta$$

$$\sigma_{y'} = \frac{\sigma_x + \sigma_y}{2} - \left(\frac{\sigma_x - \sigma_y}{2}\right) \cos 2\theta + \tau_{xy} \sin 2\theta$$

$$\tau_{x'y'} = -\left(\frac{\sigma_x - \sigma_y}{2}\right) \sin 2\theta + \tau_{xy} \cos 2\theta$$

$$\theta = \frac{1}{2} \tan^{-1} \left(\frac{2\tau_{xy}}{\sigma_x - \sigma_y} \right)$$

$$I_{xx} = \int y^2 dA, I_{yy} = \int x^2 dA, I_{xy} = \int xy dA, I_{xx'} = I_{xx} \cos^2 2\theta - I_{xy} \sin 2\theta + I_{yy} \sin^2 2\theta$$

3-Dimensional

Determinants Equations (Stress Invariants)

$$I_1 = \sigma_x + \sigma_y + \sigma_z$$

$$I_2 = \sigma_x \sigma_y + \sigma_x \sigma_z + \sigma_y \sigma_z$$

$$I_3 = \sigma_x \sigma_y \sigma_z + \tau_{xy} \tau_{yz} + \tau_{yz} \tau_{zx} + \tau_{zx} \tau_{xy}$$

$$I_4 = \sigma_x \tau_{xy} \tau_{yz} + \sigma_y \tau_{yz} \tau_{zx} + \sigma_z \tau_{zx} \tau_{xy} - \tau_{xy}^2 \tau_{yz} - \tau_{yz}^2 \tau_{zx} - \tau_{zx}^2 \tau_{xy}$$

$\sigma_{1,2,3}$ = roots of $\lambda^3 - I_1 \lambda^2 + I_2 \lambda - I_3 = 0$

$$\tau_{max} = \frac{\sigma_1 - \sigma_3}{2}$$

See ex. for principal direction

$$I_{xx'} = I_{xx} \cos^2 \theta - I_{xy} \sin 2\theta + I_{yy} \sin^2 \theta$$

$$I_{yy'} = I_{yy} \cos^2 \theta - I_{xy} \sin 2\theta + I_{xx} \sin^2 \theta$$

$$I_{xy'} = I_{xy} \cos 2\theta + \frac{I_{xx} - I_{yy}}{2} \sin 2\theta$$

Centroidal Bending

$$\sigma_{max} = \sum \frac{M y}{I} + \frac{P}{A}$$

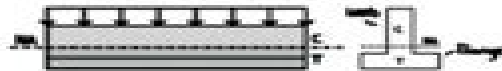
C is distance from NB to point of interest; it differs for circular shaft

I & A will be the same for both

bending stress will be the same (symmetrical axis)

NOA is used (symmetrical) stress distribution

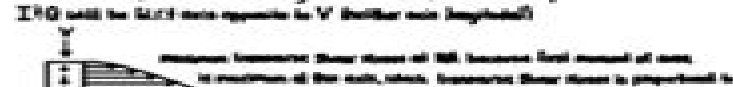
tensile bending → positive, compression bending → negative



Shear Stress

$$\tau = \frac{VQ}{It}$$

Q → Area, for rectangles along axis but I & Q are with respect to I & Q will be the same opposite to V (another axis (symmetrical))

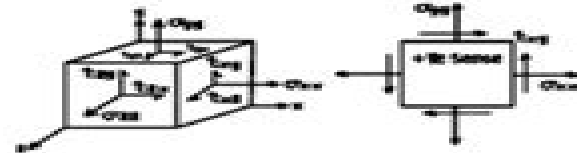
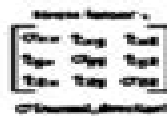


shear stress is zero at the top and bottom surfaces because first moment of area is maximum at the axis, which separates shear stress is proportional to

shear stress is zero at the top and bottom surfaces because first moment of area is maximum at the axis, which separates shear stress is proportional to

Torsion

$$\tau = \frac{T \rho}{J}$$



Case Sectional Properties

$$Centroid: \bar{y} = \frac{\sum y_i A_i}{\sum A_i}$$

Second Moment of Area

$$I = \sum I_{xxi} + \sum A_i d_i^2$$

$$d_i = (y_i - \bar{y})$$

If I_{xy}, have that in face perspective of y₁-axis

If I_{xy}, " " z₁-axis

Area Moment of Inertia

$$J = \sum (I_{xxi} + I_{yyi})$$

I_x = I_{xx} if bar is used as a shaft, hollow one: $\frac{\pi}{32} (d_o^4 - d_i^4)$

Equal Moment of Area

moment of area about or below point of interest

$$Q = A \bar{y}$$

where A is the remaining case sectional area about or below that point

and \bar{y} is the distance between the centroid of that area to the centroid of the entire case section

2-Dimensional Plane Stress & Strain

Stress

(1) — one dimension compared to other two dimensions is very small

(2) — like a plate or a shell, stress with large constant

(3) — total has to be in the plane of the plate, etc., same way it have max.

Strain

(1) — one dimension compared to other two dimensions is very large

(2) — symmetric loading condition

Stress-Strain

A: point of interest

$$\begin{bmatrix} \sigma_x \\ \sigma_y \\ \tau_{xy} \end{bmatrix} = \begin{bmatrix} C_{11} & C_{12} & C_{13} \\ C_{21} & C_{22} & C_{23} \\ C_{31} & C_{32} & C_{33} \end{bmatrix} \begin{bmatrix} \epsilon_x \\ \epsilon_y \\ \gamma_{xy} \end{bmatrix} \rightarrow \begin{bmatrix} E_{11} & E_{12} & E_{13} \\ E_{21} & E_{22} & E_{23} \\ E_{31} & E_{32} & E_{33} \end{bmatrix} \rightarrow \begin{bmatrix} E_{11} & \nu_{12} E_{11} & \nu_{13} E_{11} \\ \nu_{21} E_{11} & E_{22} & \nu_{23} E_{11} \\ \nu_{31} E_{11} & \nu_{32} E_{11} & E_{33} \end{bmatrix}$$

Stress-Strain Relationship

$$E_{xx} = E \left(\frac{\sigma_x}{E} + \nu \frac{\sigma_y}{E} + \nu \frac{\sigma_z}{E} \right) - \nu \frac{\sigma_x}{E} - \nu \frac{\sigma_y}{E} - \nu \frac{\sigma_z}{E}$$

$$E_{yy} = E \left(\frac{\sigma_x}{E} + \nu \frac{\sigma_y}{E} + \nu \frac{\sigma_z}{E} \right) - \nu \frac{\sigma_x}{E} - \nu \frac{\sigma_y}{E} - \nu \frac{\sigma_z}{E}$$

$$E_{zz} = E \left(\frac{\sigma_x}{E} + \nu \frac{\sigma_y}{E} + \nu \frac{\sigma_z}{E} \right) - \nu \frac{\sigma_x}{E} - \nu \frac{\sigma_y}{E} - \nu \frac{\sigma_z}{E}$$

$$\tau_{xy} = E \left(\frac{\sigma_x}{E} - \nu \frac{\sigma_y}{E} \right) \nu \frac{\sigma_z}{E} + \gamma_{xy} \tau_{xy}$$

$$C = E(1+\nu)(1-\nu)$$

Plane Stress Condition

$$E_{xx} = E \left(\frac{\sigma_x}{E} + \nu \frac{\sigma_y}{E} \right) - \nu \frac{\sigma_x}{E} - \nu \frac{\sigma_y}{E}$$

$$E_{yy} = E \left(\frac{\sigma_x}{E} + \nu \frac{\sigma_y}{E} \right) - \nu \frac{\sigma_x}{E} - \nu \frac{\sigma_y}{E}$$

$$E_{zz} = -\nu \frac{\sigma_x}{E} - \nu \frac{\sigma_y}{E}$$

$$\tau_{xy} = E \gamma_{xy}$$

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Guillaume Favre



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